



- Q:) The water content of a soil remains unchanged during the entire test in-[UKPSC AE 2013 PAPER-I]
- A: Drained test
- **B: Consolidated undrained test**
- **C: Unconsolidated undrained test**
- **D: None of these**

- Q:) The suitable method of finding the shear strength of very plastic cohesive soils is by means of-
- [OPSC AE-2016 (II)/OPSC AEE 2015 PAPER-
- I/Karnataka PSC AE 2015 Paper-II]
- A: Cone test
- **B: Penetration test**
- C: Vane shear test
- **D: Torsional shear test**

- Q : ) Considerable loss of shear strength due to shock or disturbance is exhibited by:
- [GPSC AE Class 91&2) Paper-2 2017 ESE 2010]
- A: Under-consolidated clays
- **B: Normally consolidated clays**
- **C: Over consolidated clays**
- D: Organic soil

- Q:) Shear strength of a soil is its [GPSC AE (CLASS 1 & 2) 2019]
- A: Minimum resistance to shear stress just before the failure
- B: Minimum resistance to shear stresses just after the failure
- C: Maximum resistance to shear stresses just before the failure
- D: Maximum resistance to shear stresses just after the failure

- Q:) Which of the following test is not used to measure the shear strength of soil?
- [GPSC AE (class 1 & 2) 2019]
- A: Triaxial compression test
- **B: Standard proctor test**
- **C: Unconfined compression test**
- **D: Vane shear test**

- Q:) If  $\alpha$  is the angle between the direction of the failure and major principal plane, and  $\phi$  is the angle of shearing resistance. Then the relationship between this two is [TNPSC AE 2019]
- A:  $\alpha = 90^{\circ} \frac{\phi}{2}$ B:  $\alpha = 90^{\circ} + \frac{\phi}{2}$ C:  $\alpha = 45^{\circ} - \frac{\phi}{2}$ D:  $\alpha = 45^{\circ} + \frac{\phi}{2}$

- Q : ) In a Mohr's circle, the shear stress T<sub>f</sub> on the plane of maximum obliquity is [GPSC AE (CLASS 1 & 2) 2019]
- A: Less than the maximum shear stress  $T_{max}$
- **B: More than the maximum shear stress T**<sub>max</sub>
- **C: Equal to the maximum shear stress T**<sub>max</sub>
- D: Numerically equal to  $(\sigma_1 \sigma_3)/2$

Q:) A vane 20 cm long and 10 cm in diameter was pressed into soft marine clay at the bottom of a bore hole. Torque was applied gradually and failure occurred at 2000 kg/cm, the cohesion of the clay in kg/cm<sup>2</sup> is: [KPSC AE 2020]

A: 
$$\frac{5}{\pi \times 7}$$
  
B: 
$$\frac{7}{\pi \times 6}$$
  
C: 
$$\frac{12}{\pi \times 7}$$
  
D: 
$$\frac{7}{\pi \times 12}$$

- Q:) Consider the following statements with regard to Soil Testing:
- 1. The origin and pole are at the same point in a Mohr's circle
- 2. The shear stress is maximum on the failure plane
- 3. Mohr's circle drawn with data from an unconfined compression test passes through the origin
- 4. Maximum shear stress occurs on a plane inclined at 45<sup>0</sup> to the principal plane
- Which of the above statements are correct?
- [ESE 2018]
- A: 1 and 2 only
- B: 2 and 3 only
- C: 3 and 4 only
- D: 1 and 4 only

- Q : ) Statement (I) : Cohesion and angle of internal friction are shear strength parameters of soils.
- Statement (II): Cohesion is zero for pure sand and angle of internal friction is zero for pure clay.
- [ESE 2017]
- A: Both statements-I and statement-II are individually true and statement-II is the correct explanation of statement-I
- B: Both statements-I and statement-II are individually true but statement-II is NOT the correct explanation of statement-I
- C: Statement-I is true but statement-II is false
- D: Statement-I is false but statement-II is true

- **Q** : ) The stability or shear strength of fine-
- grained soils can be increased by draining them
- with the passage of direct current through
- them. This process is known as
- [ESE 2020]
- A: Electro-osmosis
- **B: Zeta potential**
- **C: Electro-chemical hardening**
- **D: Consolidation**

- Q : ) If the cohesion of a pure clay found in an unconfined compressive strength test is 1 kg/cm<sup>2</sup>, then its unconfined compressive strength in kg/cm<sup>2</sup> is [GPSC AE MARCH 2018]
- A: 0.5
- **B: 2**
- C: 1
- **D: 4**

- **Q** : ) If correct value of cohesion of highly soft
- clay is to be determines, choose the correct
- type of test that should be carried out.....
- [GPSC Poly. Tch. Lect. 2016]
- A: Field vane shear test
- **B: Triaxial shear test**
- **C: Direct shear test**
- **D: Laboratory unconfined compression test**

- Q : ) Consistency as applied to cohesive soil is an indicator of its.
- [UJVNL AE 2016]
- A: Density
- **B: Shear strength**
- **C: Moisture content**
- **D: Porosity**

- Q : ) If C is cohesion,  $\sigma$  is normal stress and  $\phi$  is angle of internal friction of soil, then coulomb's equation of shear strength (S) can be represented as [GPSC AE JUNE 2019]
- A: C = S +  $\sigma \tan \phi$
- **B: S** =  $\sigma$  + **C** tan  $\phi$
- C: C = S  $\sigma \tan \phi$
- D: S = C  $\sigma \tan \phi$

- Q : ) If drainage is permitted throughout, in a shear strength test during the application of both and
- shear stress, so that full consolidation occurs and no
- excess pore water pressures develop at any stage of the test, is known as
- [GPSC ASSISTANT PROF. 2016]
- A: Consolidated undrained test
- **B: Consolidated drained test**
- **C: Unconsolidated undrained test**
- **D: Unconfined compression test**

- **Q** : ) A soil having, **C** =  $\frac{\sqrt{3}}{2}$ ,  $\phi = 30^{\circ}$  is subjected
- to cell pressure of 1 unit. The deviator stress at failure will be
- [WBPSC AE 2003]
- A: 3 unit
- B: 4 unit
- C: 5 unit
- D: 6 unit

- Q:) Which of the followings statements is/are correct?
- [Chandigarh AE 2017]
- A: Quick condition and liquefaction of saturated sands are based on similar phenomenon
- B: Quick condition is with only earth dams
- **C: Liquefaction is possible in dry sand also**
- **D: All of the above**

- Q:) Which one of the following statement provides the best argument that direct shear test are not suited for determining shear parameters of a clay soil?
- [TNPSC AE 2018]
- A: Failure plane is not the weakest plane
- **B:** Pore pressures developed can not be measured
- C: Satisfactory strain levels cannot be maintained
- **D: Adequate consolidation can not be ensured**

- Q : ) While making the vertical excavation in soft saturated clay, soil caved in at a depth of 4 m. If the unit weight of soil is 20000 N/m<sup>3</sup>, what is the cohesion of the soil?
- [BHEL ET 2019]
- A: 26.67 kN/m<sup>2</sup>
- B: 40 kN/m<sup>2</sup>
- C: 20 kN/m<sup>2</sup>
- D: 10 kN/m<sup>2</sup>

- **Q** : ) The ratio of the horizontal effective stress
- $\sigma_{\mathbf{h}'}$  to the vertical effective stress,  $\sigma_{\mathbf{v}'}$  is termed
- as :
- [Karnataka PSC AE 2017 Paper-II]
- A: Safe bearing capacity
- **B: Coefficient of lateral earth pressure**
- **C:** Friction
- D: Optimum ratio

- Q : ) The poisson ratios of soil sample 1 & 2 are  $\mu^2$ respectively and the coefficient of earth pressure at rest for soil sample 1 and 2 are  $k_1$  and  $k_2$ respectively. If  $\mu_1/\mu_2 = 1.5$  and  $(1 - \mu_1)/(1 - \mu_2) =$ 0.875, then  $k_1/k_2$  will be [ISRO Scientist/Engineer 2014]
- A: 1.3125
- B: 1.7143
- C: 1.8213
- D: 1.9687

Q : ) Consider the following assumptions regarding coulomb's wedge theory:

- 1. There is equilibrium of every element within the soil mass of the material
- 2. There is equilibrium of the whole of the material
- 3. Backfill is wet, cohesive, and ideally elastic
- 4. The wall surface is rough
- [ESE 2018]
- Which of the above assumptions are correct?
- A: 1 and 3 only
- B: 1 and 4 only
- C: 2 and 3 only
- D: 2 and 4 only

- Q:) The correct sequence of the given parameters in descending order of earth pressure intensity is [GPSC AE Class (1 & 2) Paper – 2 2017 UPRVUNL AE 2015/ ESE 2000]
- A: Active, passive, at rest
- B: Passive, active, at rest
- C: Passive, at rest, active
- D: At rest, passive, active

- Q : ) A phreatic line is defined as the line within a dam section below which there are
- [OPSC AE Paper-II 2019]
- A: Positive equipotential lines
- **B: Atmospheric pressure**
- **C: Positive hydrostatic pressure**
- **D: Negative hydrostatic pressure**

- Q : ) A vertical retaining wall retains a C- $\phi$  backfill with a surcharge of uniform intensity q per unit area. The depth Z<sub>0</sub> where the active earth pressure is zero is given by
- [RPSC AE (GWD) 2014]
- A: q/Y B:  $\frac{2C'}{\Upsilon}$  tan  $\alpha' - q/\Upsilon$ C:  $\frac{2C'}{\Upsilon}$  tan  $\alpha' + q/\Upsilon$
- **D:**  $\frac{2C'}{\Upsilon}$  tan  $\alpha'$